

1979/49. Test pitting for road-making materials along Eddystone and Musselroe Roads, Portland Municipality

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Abstract

Four sites in the Eddystone Road-Musselroe Road area were tested for gravel resources using a backhoe. Sizing analysis of material sampled and other factors show only two sites as being worth considering for future gravel pit operations.

INTRODUCTION

This report contains the results of a test programme which was recommended in a previous report (Threader, 1979) and carried out under supervision of the writer by the Portland Municipal Council using a backhoe mounted on a David Brown 995 agricultural wheeled tractor.

AREA EXAMINED

On the basis of the previous work, four localities were selected for further examination. These were:

- (1) Eddystone Road - Little Boggy Creek area
- (2) Eddystone Road - 4 km north of Area 1
- (3) Browns Bridge area
- (4) north end of Musselroe Road

The areas examined are shown in Figure 1.

RESULTS

Area 1, test pits 1-5

Test pits 3, 4 and 5, which were dug on the western side of the vehicular track leading from Eddystone Road to Little Boggy Creek, showed promise of there being a significant gravel resource, together with a suitable fall of the land to the south for ease of operation.

Area 2, test pits 6-10

All pits, except 8, were in suitable material, indicating a probable extension of the resource surrounding the existing gravel pit.

Area 3, test pits 11-13

No gravels were intersected near the bridge, probably due to the deposition of river silt over gravel. There is evidence of shallow diggings around pit 13 and it is presumed that the thin mantle of gravel previously existing has been removed.

Area 4, test pits 14-18

There was insufficient gravel fraction in these test pits for the material to have any application in road-making. Pit 14 contained 1.8 m of clayey pebbly sand which could be blended with coarser material from elsewhere. Pit 18 was deficient in fines, but this material could be used for concrete aggregate or as a road sealant.

Presentation of results

The logs of test pits, together with gravel/sand/silt-clay proportions and the results of plasticity tests carried out on all materials containing excessive fines are shown in Table 1. The gravel/sand/silt-clay ratios are shown on a triangular diagram (fig. 2) which clearly shows the sandy, clayey nature of the material. There is no true gravel and the best materials are those described as sandy gravel. A 5-10% fines content is desirable to act as a binder. Complete sizing analyses of the material tested are given in Table 2.

DISCUSSION

It was hoped to locate some areas with several metres thickness of suitable material so that the further proliferation of broad shallow pits could be avoided. Unfortunately none were located, which may be as a result of the nature of the terrain, but could also be due to the inadequacies of the plant used. The backhoe was deficient in two respects; the reach was limited to two metres and the hydraulics were inadequate for the purpose. The machine could only dig to two metres in soft sand and significantly less where cemented layers were encountered. Consequently, some holes had to be abandoned without properly investigating the site.

RECOMMENDATION

The two areas on Eddystone Road, namely 3, 4 and 5 in Area 1 and 6-10 in Area 2 are the only sites considered worth opening up as future gravel pits.

REFERENCE

THREADER, V.M. 1979. Prospecting for gravel in the Eddystone and Musselroe Roads area. *Unpubl.Rep.Dep. Mines Tasm.* 1979/7.

[23 November 1979]

Table 1. LOGS AND MATERIAL PROPERTIES OF TEST PITS

Pit No.	AMG reference	Log	Depth (mm)	Sample No.	Gravel (%)	Sand (%)	Silt & Clay (%)	Liquid Limit	Plast-icity Index	Type of material	Use
1	EQ985592	Coarse sand Sand with scattered pebbles Sand, clayey at base Cream clay	600 900 1650 2100								Not sampled, appeared deficient in coarse fraction.
2	EQ984594	Coarse sand Peaty sandy clay Off white red mottled sandy clay	600 900 1500	2.3 2.2 2.1	29 16 9	67 55 63	4 29 28	36 46 62	13 23 33	SG VCPS VCPS	Top 600 mm suitable, remainder excessive in fines. Total depth 1.5 m of marginal quality.
		Weighted mean			18	63	19			CPS	
3	EQ983593	Peaty soil Coarse white sand Cemented coarse white sand	450) 1500)) 1650)	3	21	76	3			PS	Slightly deficient in gravel fraction. Total depth in excess of 1.65 m.
					(Could not penetrate)						
4	EQ982593	Coarse sand Cemented coarse sand Medium sand, finer at base Sandy clay (continuing)	1200) 1500)) 2400) 2700	4	33	62	5			SG	Satisfactory to 2.4 m. Could possibly include some of fines to 2.7 m.
5	EQ982592	Coarse sand, peaty, iron stained Cemented coarse sand	1500 1800	5	29	68	3			SG	Satisfactory to 1.8 m.
6	EQ968636	White sand Yellow clay with scattered grit Peaty gritty clay Coarse, yellow, clayey, sand	450)) 750) 1200)) 1800)	6	21	68	11			CPS	Marginal to satisfactory to 1.8 m.
7	EQ967637	Peaty soil Coarse white sand Peaty sand Cemented, coarse, clayey sand	150) 600) 750) 1200	7.1 7.2	46 37	50 60	4 3			SG	Satisfactory to 1.2 m. Probably a greater depth available.
								(Could not penetrate)		SG	
		Weighted mean			43	54	3			SG	

Pit No.	AMG Reference	Log	Depth (mm)	Sample No.	Gravel (%)	Sand (%)	Silt & Clay (%)	Liquid Limit	Plasticity Index	Type of material	Use
8	EQ968635	Peaty, sandy soil Sand White clay with scattered grit	300 900 1200								Not sampled, no gravel.
9	EQ966638	Soil Coarse white sand Sandy clay with bands of white clay	150 600 1500	9.1 9.2	35 44	61 50	4 6			SG SG SG	Satisfactory to 1.5 m.
			Weighted mean		37	48	5				
10	EQ969634	Peaty sandy soil White sand Peaty clayey sand Coarse clayey sand	150 600 750 1350	10.1 10.2	24 30	71 55	5 15			SG CSG	Satisfactory to 1.35 m.
			Weighted mean		28	61	11			CSG	
11	EQ915677	Soil Fine white sand Iron stained sand Fine brown, clayey sand	150 750 1050 1950	11.1 11.2	0 1	91 84	9 15			S CS	Unsatisfactory, deficient in gravel fraction.
			Weighted mean		1	87	12			CS	
12	EQ918678	Fine sand and soil cemented sand	(could not penetrate)								As above
13	EQ922681	Coarse, clean sand Peaty, sandy clay Mottled white and brown gritty clay (continuing)	600 900 1900	13.1 13.2	39 35	55 44	6 21		55	SG VCSG	Marginal quality
			Weighted mean		36	48	16			CSG	
14	EQ991762	Soil Fine white sand Peaty sand Cemented iron-stained sand Clayey sand	150 600 750 1350 1800	14.4 14.3 14.2 14.1	13 13 11 4	71 77 74 70	16 10 15 26	30 Slip Slip	11	CPS CPS CPS VCSG	Unsatisfactory due to excessive fines but may be suitable for blending with less plastic materials.
			Weighted mean		10	72	18			CPS	

Pit No.	AMG Reference	Log	Depth (mm)	Sample No.	Gravel (%)	Sand (%)	Silt & Clay (%)	Liquid Limit	Plasticity Index	Type of material	Use
15	EQ990753	Coarse sand Sandy clay	300 600	(600 mm already removed) (pit abandoned)							No gravel fraction - already stripped
16	EQ988775	White sand Fine white sand Clay (continuing)	600 1050 1650	(600 mm already removed) 16.2 16.1							Unsatisfactory - excessive fines.
					0	4	96	51	22	M	
17	EQ988773	Fine, white sand Peaty sand Cemented, iron stained sand	750) 900)) 1200)	17	1	93	6	Slip		S	Unsatisfactory, no gravel fraction.
18	EQ989777	Fine white sand Partially consolidated iron stained, cross-bedded sand Granule sand White sand (in floor of old pit)	900 1500 2100 4100 Weighted mean	18.1 18.2 18.3 18.4	0 3 11 9 6	98 96 88 91 93	2 1 1 0 1			S S PS PS PS	Unsatisfactory for road-making but suitable as sealing aggregate or for concrete making.

Table 2. SIZING ANALYSES OF SAMPLED MATERIAL

Registered No.		Sizing analysis (mm)									
		9.53	4.75	2.36	1.18	0.6	0.425	0.3	0.15	0.075	-0.075
791363-2.1	Mass %		1.5	7.7	16.8	17.9	9.9	8.4	8.4	1.5	27.9
	Cum.mass %		1.5	9.2	26.0	43.9	53.8	62.2	70.6	72.1	100
791364-2.2	Mass %		2.3	13.4	16.7	18.3	7.6	5.4	5.9	1.8	28.6
	Cum.mass %		2.3	15.7	32.4	50.7	58.3	63.7	69.6	71.4	100
791365-2.3	Mass %		6.7	22.6	20.7	22.7	10	5.6	6	2.1	3.6
	Cum.mass %		6.7	29.3	50	72.7	82.7	88.3	94.3	96.4	100
791366-3	Mass %		3.9	16.8	30.3	33.4	6.9	2.9	2.4	0.8	2.6
	Cum.mass %		3.9	20.7	51	84.4	91.3	94.2	96.6	97.4	100
791367-4	Mass %	0.2	11.3	21.2	22.1	23.7	6.7	3.5	4.2	1.5	5.6
	Cum.mass %	0.2	11.5	32.7	54.8	78.5	85.2	88.7	92.9	94.4	100
791368-5	Mass %	0.2	9.1	19.8	25.5	24.3	9.1	4.6	3.7	1	2.7
	Cum.mass %	0.2	9.3	29.1	54.6	78.9	88	92.6	96.3	97.3	100
791369-6	Mass %		3.6	17.9	23.5	19.7	7.9	5.5	7.5	3.7	10.7
	Cum.mass %		3.6	21.5	45	64.7	72.6	78.1	85.6	89.3	100
791370-7.1	Mass %	0.9	16.4	28.5	22.6	14.7	5.1	2.4	3.6	1.8	4
	Cum.mass %	0.9	17.3	45.8	68.4	83.1	88.2	90.6	94.2	96	100
791371-7.2	Mass %	0.5	8.2	28.5	24.4	16.7	5.9	4	5.7	2.9	3.2
	Cum.mass %	0.5	8.7	37.2	61.6	78.3	84.2	88.2	93.9	96.8	100
791372-9.1	Mass %		8.4	27.1	25.1	17.1	5.8	4	5.8	2.4	4.3
	Cum.mass %		8.4	35.5	60.6	77.7	83.5	87.5	93.3	95.7	100
791373-9.2	Mass %	3.5	16.8	23.3	12.3	9.2	6.2	6.4	10.1	5.6	6.6
	Cum.mass %	3.5	20.3	43.6	55.9	65.1	71.3	77.7	87.8	93.4	100
791374-10.1	Mass %		6.3	17.9	21.9	23	10.2	6.1	6.7	2.5	5.4
	Cum.mass %		6.3	24.2	46.1	69.1	79.3	85.4	92.1	94.6	100
791375-10.2	Mass %		7	23.3	21.5	10.9	4.3	4.4	8.9	4.6	15.1
	Cum.mass %		7	30.3	51.8	62.7	67	71.4	80.3	84.9	100
791376-11.1	Mass %			0.2	0.9	10	18.1	24.6	28.2	9	9
	Cum.mass %			0.2	1.1	11.1	29.2	53.8	82	91	100
791377-11.2	Mass %			0.8	4.3	10.3	13.5	18.6	25.9	11.4	15.2
	Cum.mass %			0.8	5.1	15.4	28.9	47.5	73.4	84.8	100
791378-13.1	Mass %	0.1	10.5	28.2	21	16.6	6.7	3.6	4.7	2	6.6
	Cum.mass %	0.1	10.6	38.8	59.8	76.4	83.1	86.7	91.4	93.4	100
791379-13.2	Mass %	0.1	7.1	28	17.6	8.2	3.7	3.9	7.2	2.7	21.5
	Cum.mass %	0.1	7.2	35.2	52.8	61	64.7	68.6	75.8	78.5	100
791380-14.1	Mass %		0.2	4.1	13.7	14.7	9.1	7.7	14.2	10.7	25.6
	Cum.mass %		0.2	4.3	18	32.7	41.8	49.5	63.7	74.4	100
791381-14.2	Mass %			10.8	19.6	18.8	9.5	7.3	11.6	7.4	15
	Cum.mass %			10.8	30.4	49.2	58.7	66	77.6	85	100
791382-14.3	Mass %		0.6	12.8	29.5	21	7.7	5.8	8.2	4.5	9.9
	Cum.mass %		0.6	13.4	42.9	63.9	71.6	77.4	85.6	90.1	100
791383-14.4	Mass %	0.1	0.4	12.6	24.3	17.4	7.9	7.1	10.9	3.3	16
	Cum.mass %	0.1	0.5	13.1	37.4	54.8	62.7	69.8	80.7	84	100
791384-16.1	Mass %			Trace	3	4.6	4	5.3	26.2	26.5	30.4
	Cum.mass %			Trace	3	7.6	11.6	16.9	43.1	69.6	100

Registered No.		Sizing analysis (mm)									
		9.53	4.75	2.36	1.18	0.6	0.425	0.3	0.15	0.075	-0.075
791385-16.2	Mass %				Trace	0.1	0.1	0.2	0.9	2.5	96.2
	Cum.mass %				Trace	0.1	0.2	0.4	1.3	3.8	100
791386-17	Mass %			0.6	14.1	20.9	10.2	9.5	26.6	11.8	6.3
	Cum.mass %			0.6	14.7	35.6	45.8	55.3	81.9	93.7	100
791387-18.1	Mass %			Trace	25.4	43.8	4.5	4.2	17.4	2.8	1.9
	Cum.mass %			Trace	25.4	69.2	73.7	77.9	95.3	98.1	100
791388-18.2	Mass %	0.8	2.5	0.2	6.2	27.3	11.2	12.2	34.1	4	1.5
	Cum.mass %	0.8	3.3	3.5	9.7	37	48.2	60.4	94.5	98.5	100
791389-18.3	Mass %			11.1	39.1	17.2	5.9	7.4	16.6	1.6	1.1
	Cum.mass %			11.1	50.2	67.4	73.3	80.7	97.3	98.9	100
791390-18.4	Mass %	1	2	6.1	19.7	13.8	4.8	7	40.6	4.9	0.1
	Cum.mass %	1	3	9.1	28.8	42.6	47.4	54.4	95	99.9	100

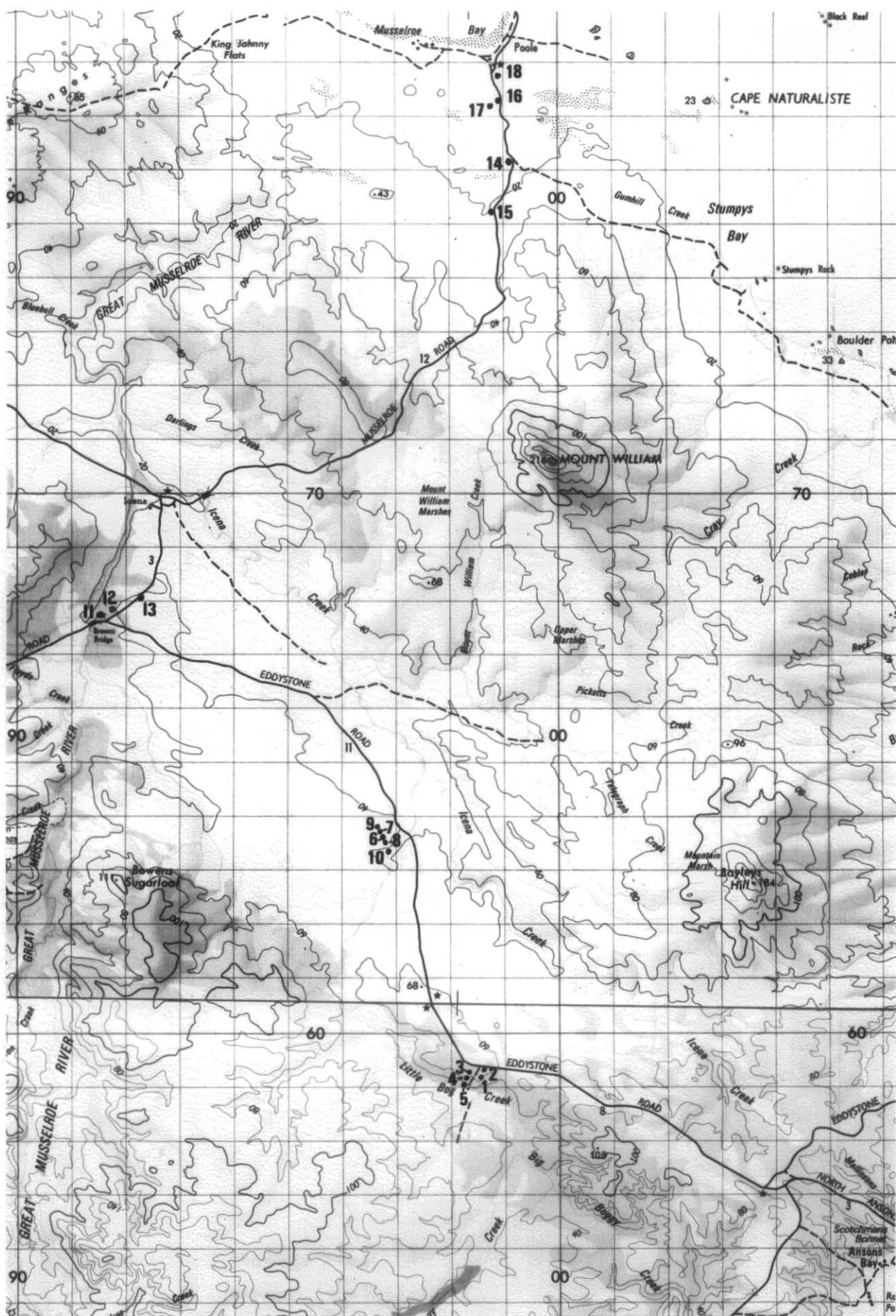
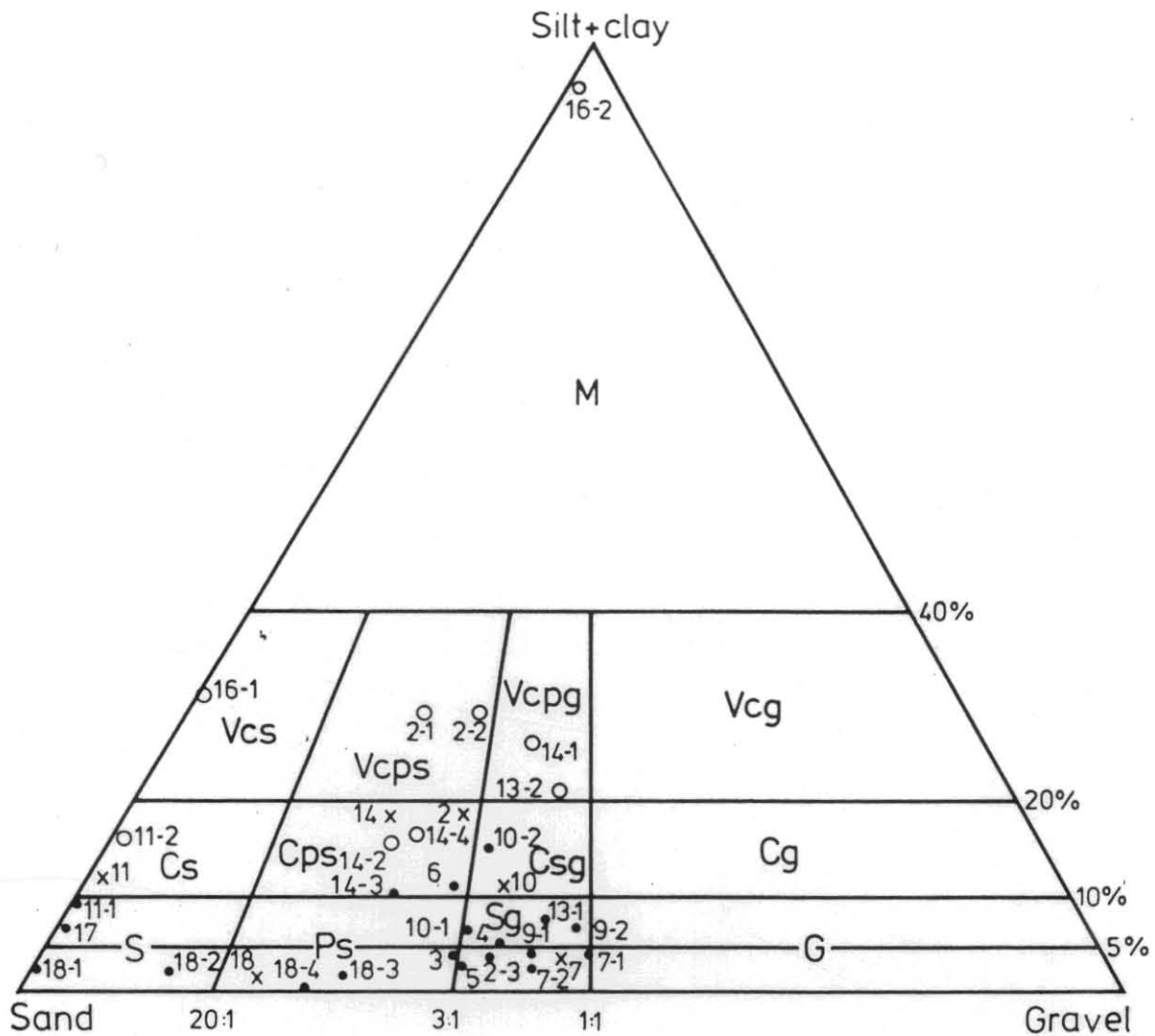


Figure 1. Location of test areas



M	mud	Csg	clayey sandy gravel
Vcs	very clayey sand	Cg	clayey gravel
Vcps	very clayey pebbly sand	S	sand
Vcpg	very clayey pebbly gravel	Ps	pebbly sand
Vcg	very clayey gravel	Sg	sandy gravel
Cs	clayey sand	G	gravel
Cps	clayey pebbly sand		

x weighted mean

o sample - Atterberg limit tests performed

• sample - sizing analysis only

5 cm

Figure 2. Grain size distribution of samples