1987/20. Subsurface examination of proposed lots, Pleasant Hills Subdivision, Rosevears.

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Abstract

Subsurface investigation of proposed building lots at Pleasant Hills subdivision has been undertaken. Slickensided clays in several test pits indicate past mass movements. A sample taken from a pit was tested to provide parameters for stability analysis. The analysis indicates that the long term stability of the clay is suspect.

INTRODUCTION

Slope classification at the proposed extension of the Vos Nominees subdivision Grindelwald (Weldon, 1986) identified areas which are at risk with respect to slope instability. A suggested alteration to the subdivision layout would have minimised the lots affected by these areas. This suggestion was unacceptable to the developer as it was at the expense of jeopardising several good lots in favour of several which, in all likelihood, would not be as readily saleable.

Detailed investigation of the lots at risk (i.e. those where the natural slopes exceeded 8°) was requested by the developer. This report details the results of the investigation.

INVESTIGATION

Eleven test pits were excavated and logged on 12 December 1986. The pits were dug by a Domino DIG tracked mini-excavator. The locations of the test pits are shown on Figure 1. The engineering logs prepared from the exposures are attached as Appendix 1.

The test pits revealed that the near-surface materials consist of slope debris derived from the weathering and erosion of Tertiary age basalt mixed with Tertiary age clayey sediments. The majority of the pits showed no evidence of disturbance nor the presence of weak materials, and do not give concern with respect to slope instability. The exceptions are test pits 1-4 which revealed fissured clays, often showing slickensides in many directions.

LABORATORY TESTING

A typical sample of the slickensided clay from 1.5-1.9 m depth in test pit 2 was submitted for laboratory tests. The results are detailed below:

Atterberg limits: Liquid Limit (%) 112 Plastic Limit (%) 24

Linear shrinkage (%) 26

Angle of internal friction (degrees) 18 Residual cohesive strength (kPa) 1.4

The angle of friction and the residual cohesive strength were determined by cyclical shearing in a shear box. As the sample was already disturbed, it is appropriate to use these values in stability analysis.

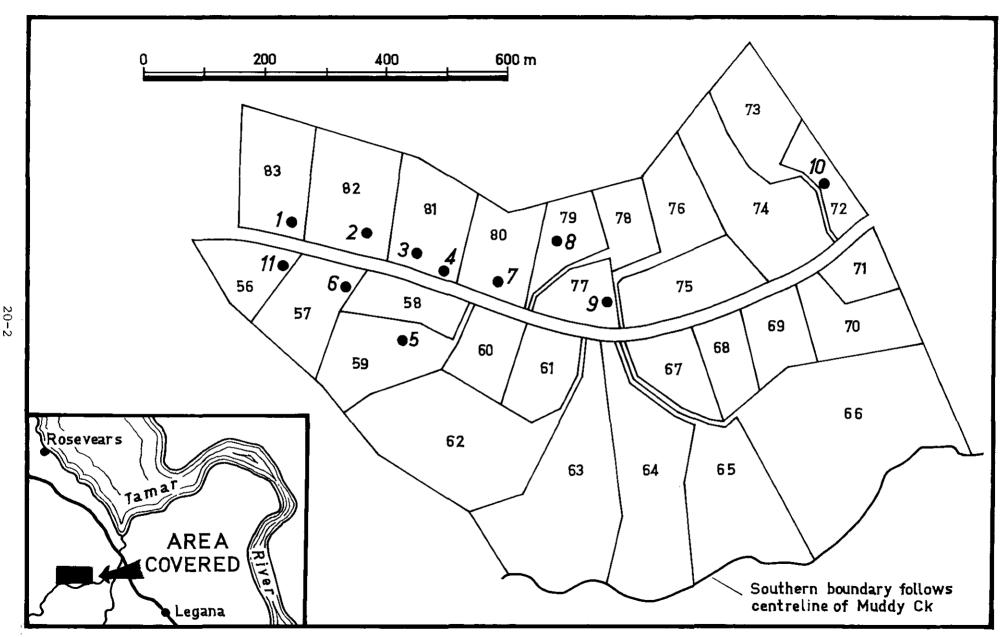
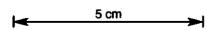


Figure 1. Locations of test pits, Pleasant Hills subdivision.



STABILITY ANALYSIS

A typical cross-section (fig. 2) in the vicinity of test pits 1-4 was selected for stability analysis using Bishop's method of slices (Bishop, 1955). The arbitrarily chosen end points of the circle were, on the downslope side, the outer edge of the roadway through the subdivision and, on the upslope side, the change in slope between the steep escarpment of the basalt plateau and the slope on which development is proposed.

An initial slope stability analysis was made for a dry slope with the residual cohesive strength and angle of internal friction as determined in the laboratory tests. A density (expressed in kN/cm^3) of 20 was selected for the soil and the effect of varying the radius of the slip circle was determined (fig. 3). As the radius increases, i.e. as the thickness of the sliding body actually decreases, the factor of safety decreases to a low point over a large range of radii (Yc between 210-300 metres).

A value of Yc=210 was adopted for subsequent slope stability analysis to determine the sensitivity of the factor of safety to density, cohesion and angle of internal friction. It was found that the stability analysis was relatively insensitive to variations in density (fig. 4) but sensitive to variations in cohesion (fig. 5) and most sensitive to changes in the angle of internal friction (fig. 6). The effect of pore water pressure ratios (Ru) is seen in Figures 5 and 6 where Ru of 0.0 indicates dry ground and Ru of 0.5 indicates virtually saturated ground.

INTERPRETATION

In stability analysis, a factor of safety equal to unity indicates a slope in delicate equilibrium, for failure is deemed to have occurred when the factor of safety falls below one. Stability analyses can, at best, only approximate actual ground conditions and require that certain assumptions be made. It is thus possible to obtain a factor of safety less than unity for slopes which have not yet failed. Conversely it is possible to obtain a factor of safety in excess of 1.0 for slopes which have actually failed.

It is generally accepted that slopes which yield a factor of safety between 1.0 and 1.3 during stability analysis have a high risk of slope failure. Moderate risks are associated with slopes where the factor of safety is determined between 1.3 and 1.5. Where the factor of safety exceeds 1.5, the slope has a low risk of slope failure.

At Pleasant Hills the factor of safety of a slope which contains slickensided clay consistently falls below 1.3 over a range of conditions when dry. The factor of safety is reduced even further should the clay be allowed to wet up.

CONCLUSIONS AND RECOMMENDATIONS

The subsurface materials on the majority of lots examined at the Pleasant Hills proposed subdivision for Vos Nominees show no evidence of disturbed or weak materials and are suitable for subdivision.

Slickensided clays encountered on lots 81-83 are evidence of past mass movement. Laboratory tests reveal low values of residual cohesive strength and angle of internal friction. A slope stability analysis of these lots indicates that a high risk of slope failure is associated with these lots. It is concluded that these lots are not suitable for development.

It is recommended that approval be given for subdivision of lots 56 to 80 at the Vos Nominees subdivision, Grindelwald.

It is recommended that approval not be given for subdivision of lots 81-83 at the Vos Nominees subdivision. Grindelwald.

REFERENCES

BISHOP, A. W. 1955. The use of the slip circle in the stability analysis of slopes. Geotechnique 5(1):7-117.

WELDON, B. D. 1986. Slope stability at a proposed subdivision at Rosevears. *Unpubl. Rep. Dep. Mines Tasm.* 1986/76.

[16 March 1987]

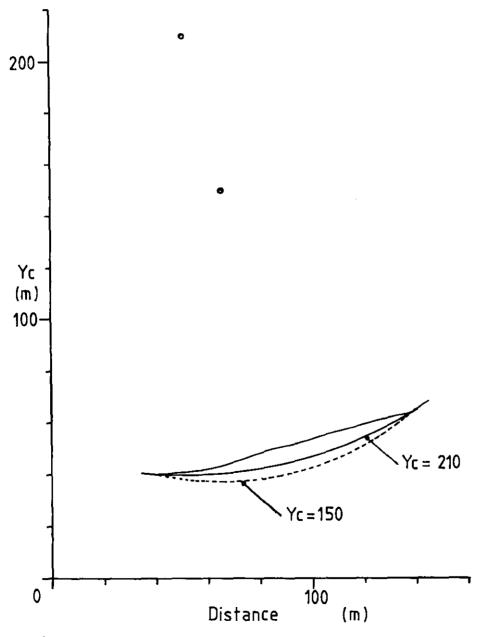


Figure 2. Cross-section for stability analysis.

STABILITY ANALYSIS

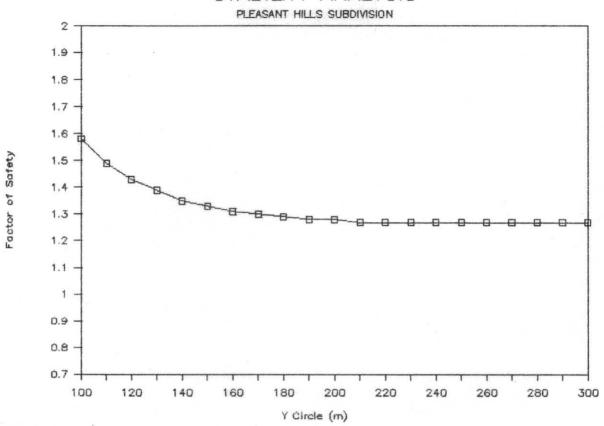


Figure 3.

PLEASANT HILLS SUBDIVISION

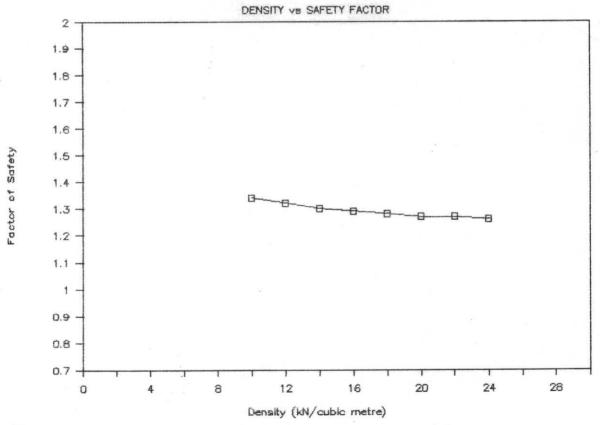


Figure 4.

5 cm

PLEASANT HILLS SUBDIVISION

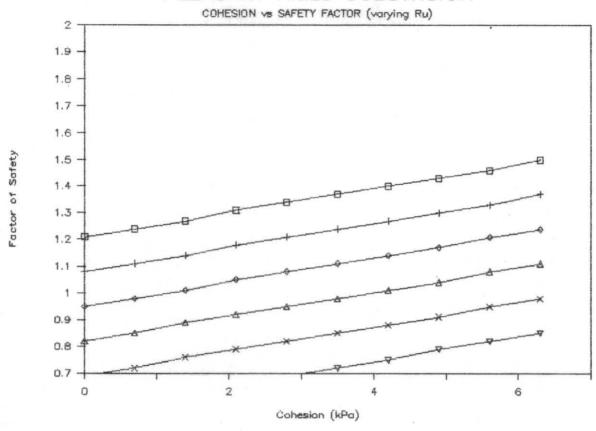


Figure 5.

PLEASANT HILLS SUBDIVISION

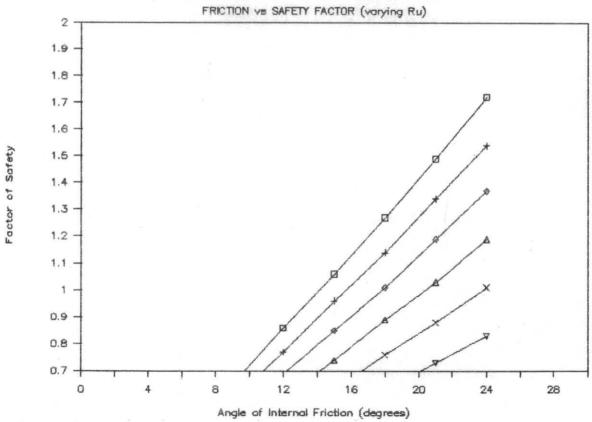


Figure 6.

EXPLANATION SHEET FOR ENGINEERING LOGS

Borehole and excavation log

Penetration Water Notes - samples and tests Material classification 123 U50 Undisturbed sample Based on Unified Soil Classification System. No resistance 22 Jan, 80 Water level on date shown. 50mm diameter. In Graphic Log materials are represented by clear contrasting symbols consistent for each project. Disturbed sample. ranging to Water inflow. Standard penetrometer blow count for 300mm. refusal Water outflow. SPT + sample.

Mo	isture content	Cons	sistency	hand penetrometer	Den	sity index	
D	Dry, looks and feel dry.	VS	Very soft.	(kPa) < 25	VL	Very loose.	% 0 - 15
M	Moist, no free water on hand when remoulding.	S	Soft.	25 - 50	L	Loose.	15 - 35
W	Wet, free water on hand	F	Firm.	50 - 100	MD	Medium dense.	35 - 65
	when remoulding.	St	Stiff.	100 - 200	D	Dense.	65 - 85
LL	Liquid limit.	VSt	Very stiff.	200 - 400	VD	Very Dense	85 - 100
PL	Plastic limit.	Н	Hard.	> 400		very belise	03 - 100
PI	Plasticity Index.	Fb	Friable.				
eg.	M > PL - Moist, moisture content greater than the plastic limit.	Notes	: X on log is te	st result of results.			

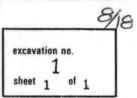
Cored borehole log

Case - lift	Fluid loss	Lugeons	Graphic log
Casing used. Barrel withdrawn.	No loss 50% loss 100% loss.	Lugeon units (µL) are a measure of rock mass permeability. For a 46 to 74mm diameter borehole 1 Lugeon is defined as a rate of loss of 1 litre per metre per minute. 1 Lugeon is roughly equivalent to a permeability of 1 × 10 → mm/sec.	No core. Rock substances represented by clear, contrasting symbols consistent for each project.

Weat	thering	Stre	ength	point load strength index Is (50) (MPa)	Significant defects
Fr	Fresh.	EL	Extremely low.	< 0.03	Significant defects shown graphically.
SW	Slightly weathered.	VL	Very low.	0.03 - 0.1	IX I
HW	Highly weathered.	L	Low.	0.1 - 0.3	Joint.
EW	Extremely weathered.	М	Medium.	0.3 - 1	Sheared zone.
		н	High	1 - 3	Crushed seam.
*		VH	Very high.	3 - 10	Infill seam.
		EH	Extremely high.	>10	Extremely weathered seam.

Note: X on log is test result.

ENGINEERING LOG – EXCAVATION



co-ordin R.L. excavation		mensions	4m x	0.6	m	exposure type Eesk pik equipment DOMINO DIG operator	pit cor pit cor logged check	nplete I by	1 18	· 12.86 · 12.86 Weldon
support	water	notes samples, tests	metres depth	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour secondary and minor components	moisture condition	consistency density index	hand penetr- ometer kPa \$200.004	structure, geology
		END	0.15	0.6000000000000000000000000000000000000	G¢.	TOPSOIL dark gray-brown silty clay, med-high plasticity, roots frootlets. GRAVELLY CLAY red-brown, medium-high plasticity, rounded medium size basalt gravel (& 40%). Moisture Content less han plastic limit CLAY red-brown, high plasticity, some medium to rootse size basalt gravel, moisture Content (M.C.) approximately equals Plastic Limit (P.L.).	n	Fl		CLAY 33 FISSULED WITH dark lustrous Storning on Some fissure peds general Very small (<10mm)
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co-ord	dinate						SUBDIVISION location Atkinsons R exposure type East bit equipment DOMINO DIG operator							nmenc npleted	18.12.86 B.Weldon				
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excavation no.
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sketch					5 cm			

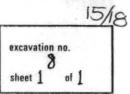
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ENGINEERING LOG – EXCAVATION

excavation no. 7 sheet 1 of 1

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ENGINEERING LOG – EXCAVATION



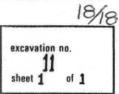
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ENGINEERING LOG – EXCAVATION



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