#### URI987-31

1987/31. Stability of land at Norwood.

W. L. Matthews

#### **Abstract**

Land comprising a proposed subdivision near the junction of Penquite and Quarantine Roads, Launceston is underlain by Tertiary sediments consisting of clay, silty clay, sandy clay and gravelly beds. The clay-rich beds usually have a high plasticity and a high linear shrinkage. Three residual strengths obtained from borehole samples resulted in low values. As the land has slope angles up to about 20°, there is risk of unstable conditions developing. Landslips occur at nearby locations. Taking the above into consideration and from stability analysis calculations, it is suggested that slopes above about 8° would be risky to develop. Development of land with this slope angle or lower should be undertaken with care.

#### INTRODUCTION

A request was made by Dale P. Luck and Associates to investigate the stability of about two hectares of land near the junction of Quarantine and Penquite Roads, Launceston. The property is owned by Mr B. L. Cordell, who is preparing to subdivide it into 13 lots. The land has been assigned mainly to classes 3 and 4 on the landslip zone map of the Tamar Valley. Much of the land in these classes, particularly the class 4 parts, is regarded as marginal for development, and detailed study is required to assess the likely future stability. Drilling has been undertaken to examine the nature of the material underlying the site. This has also allowed for the collection of samples for testing and for an examination of groundwater conditions.

#### RELIEF AND GEOLOGY

Much of the land is moderately steep, with slopes ranging up to about  $20^{\circ}$ . Flatter areas are present around the margins of the land near the two roads, and the land also flattens a little towards the top of the property. No definite features indicating landslips have been identified but there is a flattish area behind the existing house which may represent an old landslip. Landslips, both old and recent, are present on nearby slopes.

From an examination of surface exposures and nearby road cuttings it is apparent that the whole property is underlain by sediments of Tertiary age. This has been confirmed by the drilling. These sediments consist of clay, silty clay, sandy clay sand, and some gravel. Gravel occurs on the plateau to the west of the land.

#### DRILLING

Thirteen holes have been drilled on the land during two periods. Holes 1-6 were drilled on 12-13 November 1986, while Holes 7-13 were drilled on 3-4 February 1987. The holes were augered to depths of up to about eight metres. After examination of the disturbed samples from the auger drilling, a few sites were selected to drill a further hole nearby for the collection of undisturbed samples for strength testing over a few intervals of interest, and also to inspect the texture of the *in situ* material. Logs of the drill holes are appended.

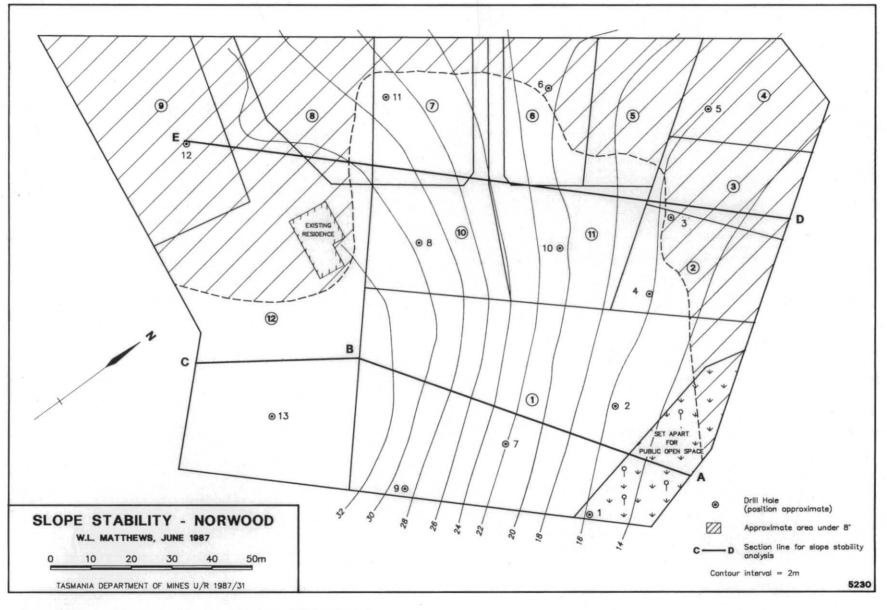
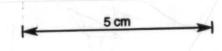


Figure 1.



The majority of sediments underlying the property are fine grained, i.e. clay and silty clay. Gravel beds prevented deep drilling in Holes 3 and 4 while Hole 12 consisted dominantly of sandy and gritty clay. Clay rich in carbonaceous matter was encountered in Holes 7 and 10.

#### SOIL TESTING

The properties of a range of materials are shown in Table 1. Plasticity is high for the majority of the samples and linear shrinkage is also high.

Three values of residual strength are shown in Table 1. These have been determined from shear box testing. The samples tested represent some of the softer materials encountered in the drilling. The sample from Hole 1 is a grey plastic clay; from Hole 7 it is a carbonaceous clay; while from Hole 13 the sample tested was a light grey to cream coloured plastic clay.

The strength factors determined indicate a low strength for these particular samples. Similar values could be expected for material in other holes at selected intervals.

Table 1. SOIL TESTING RESULTS

					Clay	fractio (%)	n XRD		
Hole no.	Depth (m)	LL	PL	LS	К	М	G	ø'	c'(kPa)
1	2.4-4.9	76	21	17					
1	6.1-6.7	125	28	23	75	20	5	10°	4.1
2	0.6-1.4	118	25	22					
2	1.4-3.2	104	22	21					
2	3.2-4.0	87	23	19					
4	0.3-1.4	104	21	23					
5	3.5-4.7	85	19	20					
6	1.8-3.2	83	21	18					
6	3.2-3.7	67	18	16					
6	5.0-7.8	102	23	20					
7	6.4	100	28	19	75	15	10	9°	6.4
10	0.9-1.8	98	23	20					
10	1.8-4.3	107	24	21					
10	4.3-7.2	120	26	22					
11	1.8-2.1	92	26	19					
13	4.6-5.5	91	22	20				10°	1.5

Determinations by R.N. Woolley, Department of Mines, Rosny Park K = kaolinite. M = montmorillonite, G = gibbsite

#### DISCUSSION OF STABILITY

To aid in the assessment of the future stability, the strength values determined above have been applied to various trial slips in sections drawn across the land. From examination of many recent slips in surrounding areas most slips extend only to relatively shallow depth  $(3-5\ m)$  but deeper slips should not be discounted. Bishops method of slices has been used in the analyses.

Trial slips on the slope ABC result in a very low safety factor for a small segment of the slope as well as for the whole slope. It is obvious that the values used do not occur for the whole length of the theoretical slip surface, otherwise the slope would be unstable at present (see attached analyses). Similarly for a portion of a slope D-E using higher cohesion values, the factor of safety reaches satisfactory levels with very low pore pressure, but in the more dangerous periods, i.e. during extended wet periods, saturation may occur for at least half of the trial slip depth. Calculated factors of safety are too low to ensure stability with certainty when this is considered.

The strength values have been applied to constant slopes of 10° and 8°. Low values for factors of safety are again calculated for 10° slopes under most of the values used. Only when the pore pressure is very low are safety factor values at reasonable levels (above 1.3) but these may be unrealistic situations for very wet periods. Reasonable values for safety factors are calculated for the 8° slope over the range of values that are most likely to prevail.

The values calculated are likely to apply over short parts of these slopes at least (not the whole slope, otherwise it would be in continuous failure). Development which includes excavation, loading and alteration of drainage will alter the present situation. Because of the presence of unstable conditions on some surrounding land which appears to have similar material, it is suggested that development only take place on areas where slopes are no greater than 8°. This excludes large portions of the steeper land. Even on slopes of 8° or less particular attention will have to be given to ensuring excellent drainage and to the limiting or the strong support of excavations. Disturbance of the nearby steep slopes should be avoided as much as possible, and tree planting should be encouraged.

#### CONCLUSIONS

Drilling and strength testing has shown that the proposed subdivision is underlain by materials with low strength. This confirms that the situation is similar to that of nearby areas where testing has been undertaken and where signs of landslips have been noted.

Taking the surrounding signs of past instability into consideration and the stability analyses for this area, it is apparent that the steeper land has a risk of becoming unstable if developed. For this reason it is recommended that areas with slopes of greater than 8° remain undeveloped.

On developing the flatter land extreme care will need to be taken to ensure excellent drainage. Excavations, unless retained by strengthened drained structures, shold be kept to a minimum. Excavations on nearby steep slopes should be avoided as far as possible. Tree planting would aid in maintaining the stability of the steeper land.

[23 June 1987]

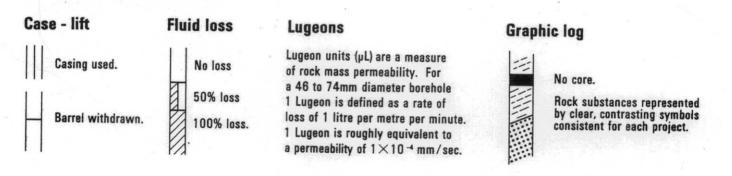
#### **EXPLANATION SHEET FOR ENGINEERING LOGS**

#### Borehole and excavation log

#### Penetration Water Notes - samples and tests Material classification 123 U50 Based on Unified Soil Classification System. No resistance Undisturbed sample 22 Jan, 80 Water level 50mm diameter. on date shown. In Graphic Log materials are represented by clear contrasting symbols consistent for each project. D Disturbed sample. ranging to Water inflow. Standard penetrometer refusal blow count for 300mm. Water outflow. N \* SPT + sample.

Mo	isture content	Cons	sistency	hand penetrometer	Den	sity index	%
D	Dry, looks and feel dry.	VS	Very soft.	(kPa) < 25	VL -	Very loose.	70 – 15
M	Moist, no free water on hand when remoulding.	S	Soft.	25 - 50	L	Loose.	15 - 35
W	Wet, free water on hand	F	Firm.	50 - 100	MD	Medium dense.	35 - 65
	when remoulding.	St	Stiff.	100 - 200	D	Dense.	65 - 85
LL	Liquid limit.	VSt	Very stiff.	200 - 400			
PL	Plastic limit.		very still.	200 - 400	VD	Very Dense	85 - 100
PI	Plasticity Index.	H	Hard.	> 400			
		Fb	Friable.				
eg.	M > PL - Moist, moisture content greater than the plastic limit.	Notes	: X on log is te	st result of results.			_T/

#### **Cored borehole log**



Weat	hering	Stre	ngth	point load strength index I <sub>5 (50)</sub> (MPa)	Significant defects
Fr	Fresh.	EL	Extremely low.	< 0.03	Significant defects shown graphically.
SW	Slightly weathered.	VL	Very low.	0.03 - 0.1	KI
HW	Highly weathered.	L	Low.	0.1 - 0.3	Joint.
EW	Extremely weathered.	M	Medium.	0.3 - 1	Sheared zone.
		Н	High	1 - 3	Crushed seam.
		VH	Very high.	3 - 10	Infill seam.
		EH	Extremely high.	>10	Extremely weathered seam.
		Note:	X on log is test resu	lt.	

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			2-		C14	SILTY CLAY - darker brown grey, friable occasional quarty fragments	n			"
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	James 1	ATT.	7-	これになった	014	CLAT - dark grey plastic, organic with	M-W		•	1q
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	ATT	2-	111111		CLAY - light brown and frey, mottled, plastic, fairly soft.	M-W			ч
		3-	121.11	CH	CLAT - gry and brown mottled, Plastice moderately soft	M-4			4
Nove	ATT	4-	(4)3141	CH	CLAY- fory, plastic, fairly soft, becoming silty	A			
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					N. C. Shin	CH	CLAY - grey brown and reddish plastic	m			•
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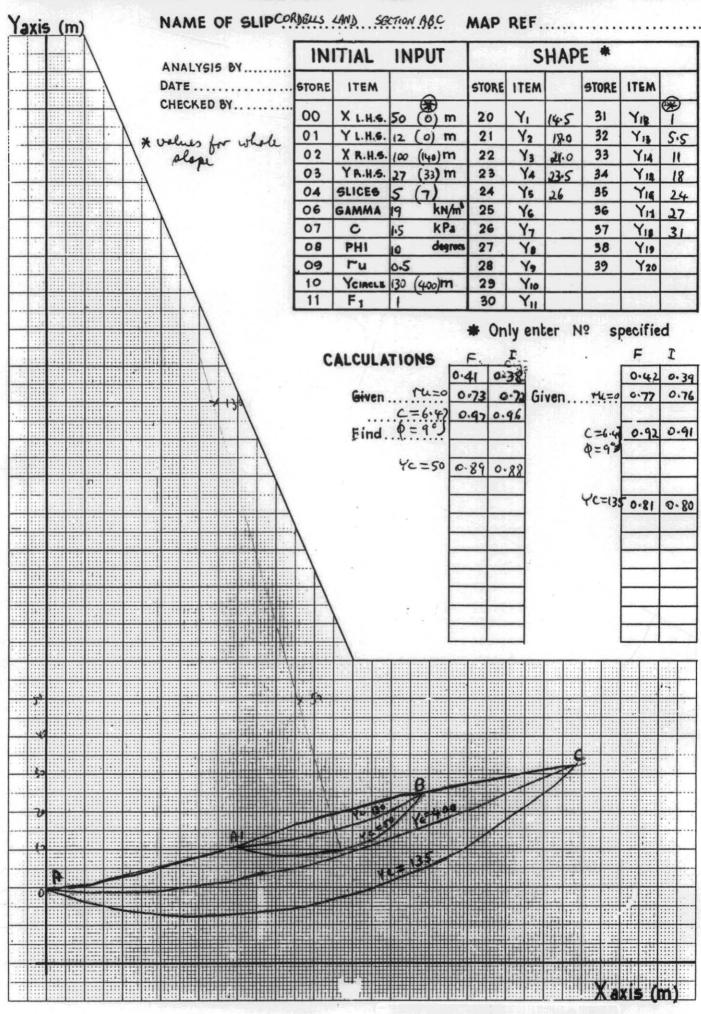
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			ATT	5-		福温温	CLAY-silly, brown plastic, becoming finer frames (less sill) towards beare	4			4			
		4	6'	7-		果里的高温度的影	CLAY - light frey brown to yellow, plastic, some zones with a little silt	~			H			
		-		8-		1111	CLAY - reddith and brownish mottled, plastic.	m			ч			
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## SLOPE STABILITY ANALYSIS - COMPUTATIONS SHEET 1 OF .....



# SLOPE STABILITY ANALYSIS - COMPUTATIONS SHEET 1 OF ....

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